

How to make UMATs for soils in ABAQUS

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Introduction

Example:

MODIFIED CAM-CLAY Plasticity constitutive equation:

$$\mathbf{T} = \mathbb{C}^{ep} : \boldsymbol{\varepsilon} \quad (1)$$

with $\mathbb{C}^{ep} =$

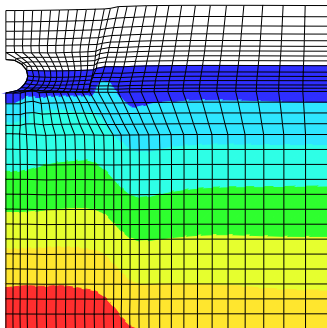
$$\begin{cases} \mathbb{C}^e, & \gamma = 0 \\ \mathbb{C}^{ep} = \mathbb{C}^e - \frac{1}{\chi} \left[a_1 \mathbf{1} \otimes \mathbf{1} + a_2 \left[\vec{\mathbf{T}}^* \otimes \mathbf{1} + \mathbf{1} \otimes \vec{\mathbf{T}}^* \right] + a_3 \vec{\mathbf{T}}^* \otimes \vec{\mathbf{T}}^* \right], & \gamma > 1 \end{cases}$$

where a_1, a_2, a_3 and χ are function of $\mathbf{T}$ and material constant, and

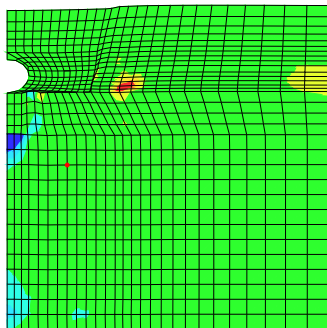
$$\vec{\mathbf{T}}^* = \frac{\mathbf{T} - \frac{1}{3} \mathbf{1}}{\| \mathbf{T} - \frac{1}{3} \mathbf{1} \|} \quad (2)$$

You make an UMAT and build a FEM model in ABAQUS!

FEM models

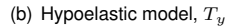


(c) Pore pressure u



(d) Shear stress T_{xy}

Figure: CAM-CLAY model

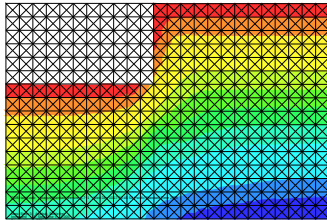


2 3 4

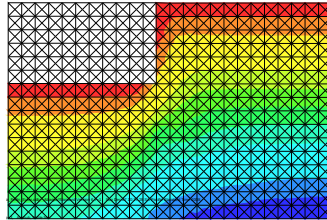
³UMATs from Fuentes.

⁴In Hypoelasticity, the RICHART's proposal is taken.

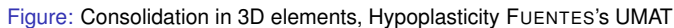
FEM models



(c) Hypoplasticity model FUENTES's UMAT, T_y



(d) Hypoplasticity model FELLIN's UMAT, T_y



INCREMENTAL DRIVER, from (NIEMUNIS, 2007)

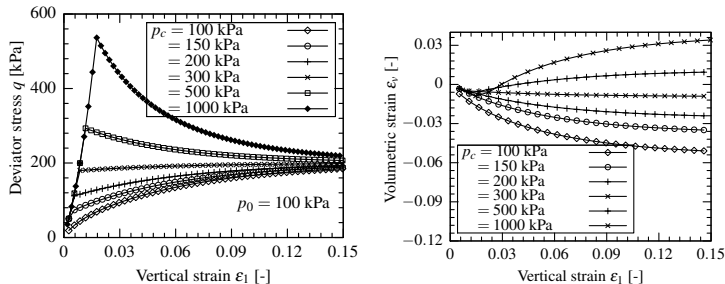


Figure: Drained triaxial path, variation of preconsolidated pressure p_c' , CAM-CLAY model

⁶For INCREMENTAL DRIVER see [Niemunis, 2008]

INCREMENTAL DRIVER, from (NIEMUNIS, 2007)

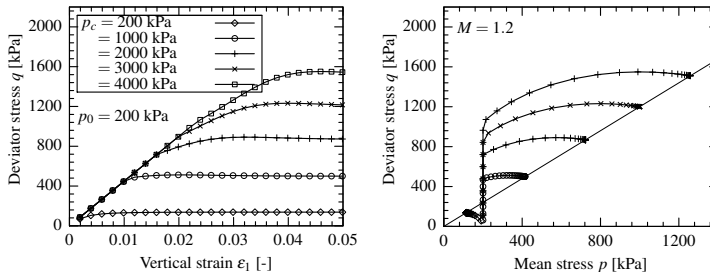
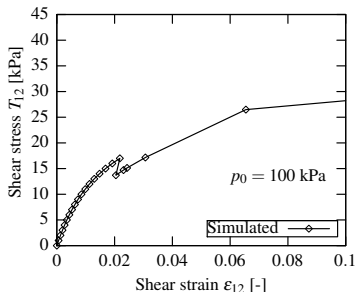
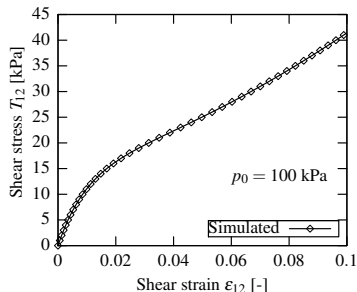


Figure: Undrained triaxial path, variation of preconsolidated pressure p_c , CAM-CLAY model

Some errors in programming



(a) FELLIN's UMAT



(b) FUENTES's UMAT

Figure: Proportional path in shear stress control in Hypoplasticity

Outline

- 1 Introduction
- 2 Programming in FORTRAN
- 3 Elastic model
- 4 Making the UMAT for elastic model
- 5 CAM-CLAY plasticity model
- 6 Hypoplasticity model
- 7 References

Programming in FORTRAN

Fortran advantages

- Developed by IBM at 1957.
- Then, versions FORTRAN IV, 77, 80 90, and 2003 were developed.
- Oriented to rapid calculations in numeric implementations.
- Not well oriented for other purposes, such as data bases.
- In some calculations, results indicates that what VISUAL BASIC takes 80 hours long, is equivalent to 10 seconds in FORTRAN.
- ABAQUS accepts FORTRAN subroutines.

More information can be encountered in [Chivers et. al., 2006].

Variable types

- **Integer:** 1, 243, -15, ...
- **Real simple precision:** 7 spaces + sign+ exponent. $1.0d2$,
 $-4.5678d-25$
- **Real double precision:** 13 spaces + sign+ exponent. $2.0d4$,
 $-4.567893456321d-25$
- **Complex double precision:** $(-123.65, 1.5E + 2) \rightarrow (\text{real}, \text{imaginary})$
- **Character:** Hello, hola
- **Logical:** .TRUE., .FALSE.

([Chivers et. al., 2006].)

Variable types

```
INTEGER a,b,c, NTENS, I, J
```

```
REAL a,b,c
```

```
Double Precision a,b,c, STRESS(NTENS), DSTRAN(NTENS), dtime  
& DDSDD(NTENS, NTENS),
```

```
LOGICAL a,b,c, info
```

```
CHARACTER*4 hola
```

```
PARAMETER (pi=3.1415926)
```

7 8

⁷in CHARACTER type 4 is the number of characters

⁸& indicates that a the line above continues in that line. Any symbol can be used.

Function and math operations

- **Power** `**`; example: `a=5.0d0**2`
- **Multiplication and division** `*` and `/`; example: `b=3.5d0/2`
- **Addition and subtraction** `+` and `-`; example: `c=4.41d0+ 5.98d0`
- **Absolute value** `ABS(X)`; example: `a=ABS(-10.0d0)`
- **Square root** `SQRT(X)`; example: `b=SQRT(17.8d0)`
- **SIN, COS, TAN** `SIN(X)`, `COS(X)`, `TAN(X)`; example: `c=SIN(1.2375d0)`
- **ASIN, ACOS, ATAN** `ASIN(X)`, `ACOS(X)`, `ATAN(X)`; example:
`c=ASIN(0.536d0)`
- **Integer conversion** `INT(X)`; example: `c=INT(12.817865d0)`
- **Real conversion** `REAL(X)`; example: `a=REAL(c)`
- **MAX, MIN** `MAX(X, , ...)`, `MIN(X, , ...)`; example: `a=MAX(1, 10, 5)`
- **Modulus** `MOD(X, Y)`; example: `a=MOD(10.5, 2) (=10.5 - INT(10.5/2))`

Data management

- **Read data** READ*; example: READ*, a, then you introduce the data for a and press enter
- **Print data** PRINT*; example: PRINT*, a; PRINT*' ,Hello World!!""
- **pause** PAUSE

Data management

- **Begin Program** PROGRAM [...name...]; example: PROGRAM [Trial],
This is the first sentence.
- **End Program** END[PROGRAM [...name...]]; example:
END[PROGRAM [Trial]]
- **Stop the program** STOP
- **Stop subroutine or function** RETURN

Exercise No 1

Open MICROSOFT VISUAL STUDIO 2008⁹ ⇒ New ⇒ Project ⇒ Intel Fortran
Project ⇒ Empty project.
Project ⇒ Add new element ⇒ Source.

⁹Intel Visual Fortran Compiler needed.

Exercise No 1.

(Compile the executable first, then run the program).

```
PROGRAM Trial
double precision a, b, c
READ*,a
READ*,b
c=sqrt(a+b)
PRINT*, c
pause
END PROGRAM Trial
```

TIPS

- Needs a Tab space before each instruction.
- Can change the source file name by right-clicking in the archive.

Logical operators

Description	Symbol	Alternative
Less than	<	.LT.
Greater than	>	.GT.
Equal than	==	.EQ.
Less or equal than	<=	.LE.
Greater or equal than	>=	.GE.
Different than	/=	.NE.

Description	Symbol
Negation	.NOT.
Conjunction	.AND.
Disjunction	.OR.

Condition IF and Cycle DO

Conditional IF,

```
IF ((a>3).AND.(a<5)) then  
b=1.0d0  
ELSEIF (a>5)  
b=2.0d0  
ELSE  
b=3.0d0  
ENDIF
```

Cycle For,

```
DO i=1,3  
    DO j=1,3  
        A(i,j)=B(i,k)*C(k,j)  
    END DO  
END DO
```

SUBROUTINES and FUNCTIONS in Fortran

SUBROUTINES for dividing the program into sub-programs.

```
SUBROUTINE dyadic(am,bm,Cm)
integer i, j
double precision am(6,1), bm(6,1), Cm(6,6)
Cm=0
do i=1,6
  do j=1,6
    Cm(i,j) =Cm(i,j)+am(i,1)*bm(j,1)
  enddo
enddo
return
END SUBROUTINE dyadic
```

In Program:

```
CALL dyadic(a,b,C)
```

SUBROUTINES and FUNCTIONS in Fortran

FUNCTIONs to obtain one single output variable.

```
REAL FUNCTION fun(x,y)
REAL x
INTEGER y
if (x.LE.(1./y.)) then
    fun=1.-y*x
else
    fun=0.
end if
END FUNCTION fun
```

In program,

a=fun(d,e)

Function RETURN for exiting the subroutine or function.

Matrix operations

Declaration:

```
double precision A(3,1), B(3,1), C(3,1)
```

Filling data:

```
data A/1.0d0,2.0d0,3.0d0/  
B=0.0d0
```

Operations:

- **Addition and subtraction** +, -; example: $A+B$
- **Component multiplication** *; example: $A=B*C$
- **Matrix Multiplication** MATMUL(,); example: $A=MATMUL(B,C)$
- **Transpose** Transpose(); example: $Transpose(A)$

Exercise No 2.

```
PROGRAM Exercise2
double precision A(3,1), B(3,1), C(3,1)
integer i
data A/1.0d0,2.0d0,3.0d0/
do i=1,3
B(i,1)=1.0d0
C(i,1)=0.0d0
end do
CALL contract(A, B, C)
PRINT*, C
Pause
END PROGRAM Exercise2
SUBROUTINE contract(A, B, C)
double precision A(3,1), B(3,1), C(3,1)
if ((B(1,1).NE.0.).AND.(B(2,1).NE.0.)) then
C(1:2,1)=A(1:2,1)/B(1:2,1)
C(3,1)=0
endif
return
END SUBROUTINE contract
```

Elastic model

Constitutive equation

$$\mathbf{T} = \left[\frac{\nu E}{(1 + \nu)(1 - 2\nu)} \mathbf{1} \otimes \mathbf{1} + \frac{E}{(1 + \nu)} \mathbb{I} \right] : \boldsymbol{\varepsilon} \quad (3)$$

where,

- $\mathbf{T}$ CAUCHY stress tensor.
- $\boldsymbol{\varepsilon}$ strain tensor (infinitesimal).
- $\mathbf{1}$ Unit second order tensor.
- $\mathbb{I}$ Unit fourth order tensor for symmetric tensors.
- E and ν material parameters.

Also, an equivalent formulation can be written (see [Lai et.al., 1993]),

$$\mathbf{T} = \left[\left(k - \frac{2}{3}G \right) \mathbf{1} \otimes \mathbf{1} + 2G\mathbb{I} \right] : \boldsymbol{\varepsilon} \quad (4)$$

with G and k material parameters.

Representation

Matrix representation chosen, takes advantage of symmetry, (Coincides with ABAQUS representation.)

$$[\mathbf{T}] = \begin{bmatrix} T_{11} \\ T_{22} \\ T_{33} \\ T_{12} \\ T_{13} \\ T_{23} \end{bmatrix}, \quad [\boldsymbol{\varepsilon}] = \begin{bmatrix} \varepsilon_{11} \\ \varepsilon_{22} \\ \varepsilon_{33} \\ 2\varepsilon_{12} \\ 2\varepsilon_{13} \\ 2\varepsilon_{23} \end{bmatrix} = \begin{bmatrix} \varepsilon_{11} \\ \varepsilon_{22} \\ \varepsilon_{33} \\ \gamma_{12} \\ \gamma_{13} \\ \gamma_{23} \end{bmatrix} \quad (5)$$

Notice the selection of shear strain. Special care to keep invariance when making matrix operations.

Representation

Tangent moduli fourth order tensor representation:

$$[\mathbb{C}^e] = \begin{bmatrix} \bar{\lambda} + 2\mu & \bar{\lambda} & \bar{\lambda} & 0 & 0 & 0 \\ \bar{\lambda} & \bar{\lambda} + 2\mu & \bar{\lambda} & 0 & 0 & 0 \\ \bar{\lambda} & \bar{\lambda} & \bar{\lambda} + 2\mu & 0 & 0 & 0 \\ 0 & 0 & 0 & \mu & 0 & 0 \\ 0 & 0 & 0 & 0 & \mu & 0 \\ 0 & 0 & 0 & 0 & 0 & \mu \end{bmatrix} \quad (6)$$

$$\text{with } \bar{\lambda} = \frac{\nu E}{(1 + \nu)(1 - 2\nu)} \text{ and } \mu = \frac{E}{2(1 + \nu)}$$

CAM-CLAY Elasticity and Hypoelasticity

CAM-CLAY Elasticity:

$$E = 3(1 - 2\nu)(1 + e) \parallel (p/\kappa) \parallel \quad (7)$$

being:

- e the void ratio.
- $p = (T_{11} + T_{22} + T_{33})/3$ the mean pressure.
- κ the compression index in semi-log space.

Hypoelasticity:

$$G = G_0 p_{atm} \frac{(2.97 - e)^2}{(1 + e) \sqrt{(\parallel p/p_{atm} \parallel)}} \quad (8)$$

being:

- G_0 initial shear modulus.
- p_{atm} atmospheric pressure.

Making the UMAT for elastic model

Description

The UMAT is an ABAQUS user's subroutine in order to define new materials constitutive model. For more information see [Hibbit et.al., 1995].

- Accepts FORTRAN and C++ subroutines.
- NEWTON's method used for numeric integration. Other methods disponibles.
- Will be called by ABAQUS for each integration point that belongs to the material defined.
- Can use and view the results of state variables.
- Temperature dependent and time dependent constitutive models can be implemented.
- Needs to provide stresses and Jacobian $\mathbb{J} = \frac{\partial \Delta \mathbf{T}}{\partial \Delta \boldsymbol{\varepsilon}}$ at the end of each increment.

Convention

The UMAT is an ABAQUS user's subroutine in order to define new materials constitutive model.

$$\begin{bmatrix} T_{11} \\ T_{22} \\ T_{33} \\ T_{12} \\ T_{13} \\ T_{23} \end{bmatrix} = \left[\begin{array}{ccc|ccc} C_{1111} & C_{1122} & C_{1133} & C_{1112} & C_{1113} & C_{1123} \\ & C_{2222} & C_{2233} & C_{2212} & C_{2213} & C_{2223} \\ & & C_{3333} & C_{3312} & C_{3313} & C_{3323} \\ \hline & & \text{sym} & C_{1212} & C_{1213} & C_{1223} \\ & & & & C_{1313} & C_{1323} \\ & & & & & C_{2323} \end{array} \right] \begin{bmatrix} \varepsilon_{11} \\ \varepsilon_{22} \\ \varepsilon_{33} \\ \gamma_{12} \\ \gamma_{13} \\ \gamma_{23} \end{bmatrix} \quad (9)$$

Dimensions can change according to the element type.

- NDI: number of direct normal components
- NSHR: number of direct shear components
- NTENS: total components = NDI + NSHR

ABAQUS variables for UMAT

Most relevant variables for UMATs for soils:

- $DDSDDE(NTENS, NTENS)$: Jacobian matrix.
- $STRESS(NTENS)$: Stress at the beginning of the increment.
- $STATEV(NSTATV)$: State variables at beginning of the increment.
- $NSTATV$: number of state variables.
- $PNEWDT$: ratio of the suggested $DTIME$ with respect to the current $DTIME$.
- $STRAN(NTENS)$: Total strains at beginning of the increment.

ABAQUS variables for UMAT

Most relevant variables for UMATS for soils:

- DSTRAN (NTENS) : Increment of strain for current iteration.
- DTIME: Increment of time.
- TIME(2) : Time at beginning and at the end of increment.
- PROPS (NPROPS) : Material parameters
- NPROPS: Number of material parameters.
- COORDS: Coordinates of current integration point.
- NOEL: Number of the current element.

UMAT syntax

Always the subroutine begins with:

```
*USER SUBROUTINES
      SUBROUTINE UMAT(STRESS,STATEV,DDSDDE,SSE,SPD,SCD,
1     RPL,DDSDDT,DRPLDE,DRPLDT,
2     STRAN,DSTRAN,TIME,DTIME,TEMP,DTEMP,PREDEF,DPRED,CMNAME,
3     NDI,NSHR,NTENS,NSTATEV,PROPS,NPROPS,COORDS,DROT,PNEWDT,
4     CELENT,DFGRD0,DFGRD1,NOEL,NPT,LAYER,KSPT,KSTEP,KINC)

C
      INCLUDE 'ABA_PARAM.INC'
      !   implicit real(8) (a-h,o-z)

C
      CHARACTER*80 CMNAME
      DIMENSION STRESS(NTENS),STATEV(NSTATEV),
1     DDSDDE(NTENS,NTENS),DDSDDT(NTENS),DRPLDE(NTENS),
2     STRAN(NTENS),DSTRAN(NTENS),TIME(2),PREDEF(1),DPRED(1),
3     PROPS(NPROPS),COORDS(3),DROT(3,3),DFGRD0(3,3),DFGRD1(3,3)
```

Parameters:

- 1 E YOUNG's modulus.
- 2 ν POISSON ratio.

State variables

- 1 Type of elasticity, 1=linear, 2=CAM-CLAY, 3=Richart.
- 2 e void ratio.
- 3 p_{atm} atmospheric pressure (RICHART'S).
- 4 G_0 initial shear modulus (RICHART'S).
- 5 κ swelling index according to CAM-CLAY.

UMAT syntax

Declare your own variables:

```
double precision T(6), DeltaE(6), ELMOD(6,6),G0, void, patm
1 K, p, max, kappa, lambda, mu
integer model
PARAMETER (ONE=1.0D0, TWO=2.0D0)
```

Initialize variables:

```
C -----
C   Initializes in fixed R6 arrays.
C   CALL Initial(NTENS, NDI, NSHR, T, STRESS, DeltaE, DSTRAN)
C   p=(T(1)+T(2)+T(3))/3.0d0
C -----
```

UMAT syntax

Establish type of elasticity:

```
C      Type of elasticity, ==1 1==linear, 2==Cam-Clay, 3==Richart
      model=STATEV(1)
C      Void ratio
      void=STATEV(2)
C      Compression index cam-clay
      kappa=STATEV(5)
C      Initial shear modulus
      G0=0.0d0
C      Atmospheric pressure
      patm=0.0d0
C      Young modulus
      E=PROPS(1)
C      Poisson ratio
      ANU=PROPS(2)
C      Maximum ratio for E or G allowed.
      max=5000.0d0
```

UMAT syntax

Establish type of elasticity:

```
C -----  
C      if (model==2) then  
C      Cam-Clay elasticity  
C      E=3.0d0*(1.0d0-2.0d0*ANU)*(1+void)*abs(p/kappa)  
C      if (E<PROPS(1)) then  
C      E=PROPS(1)  
C      endif  
C      endif
```

UMAT syntax

Establish type of elasticity (continuation):

```

C -----
    if (model==3) then
C      Hipoelasticity Richart
C      atmospheric pressure
      patm=abs(STATEV(3))
C      Initial shear modulus
      G0=abs(STATEV(4))
C      Current shear modulus according to Richart
      G=G0*patm*(2.97d0-void)**2.0d0/(1.0d0+void)*sqrt(abs(p/patm))
C      Limits of shear modulus
      if (G<G0) then
        G=G0
      endif
      if (G>max*G0) then
        G=max*G0
      endif
C      Bulk modulus and Young modulus
      K=G*2.0d0*(1.0d0+ANU)/(3.0d0*(1.0d0-2.0d0*ANU))
      E=3.0d0*K*(1.0d0-ANU)
    endif

```

UMAT syntax

Tangent moduli in 6x6 matrix:

```
lambda=ANU*E/ (ONE+ANU)/(ONE-TWO*ANU)
mu=E/TWO/(ONE+ANU)
ELMOD=0.0d0
```

```
C -----
C Elastic moduli
ELMOD(1,1)=lambda+TWO*mu
ELMOD(2,2)=ELMOD(1,1)
ELMOD(3,3)=ELMOD(1,1)
ELMOD(4,4)=mu
ELMOD(5,5)=mu
ELMOD(6,6)=mu
ELMOD(1,2)=lambda
ELMOD(1,3)=lambda
ELMOD(2,3)=lambda
ELMOD(2,1)=ELMOD(1,2)
ELMOD(3,1)=ELMOD(1,3)
ELMOD(3,2)=ELMOD(2,3)
```

UMAT syntax

Next stress in 6x1 vector

```
C      Euler's forward integration scheme
      DO I=1,NTENS
        DO J=1,NTENS
          T(I)=T(I)+ELMOD(I,J)*DeltaE(J)
        ENDDO
      ENDDO

C      Subroutine for filling the stress and Jacobian matrix
      Call Solution(NTENS, NDI, NSHR, T, STRESS, ELMOD, DDSDE)
```

The `Solution` subroutine transform the stress vector `T` and the jacobian `ELMOD` according to the real size using `NTENS, NDI, NSHR`.

UMAT syntax

State variables computations and end of subroutine UMAT

```
C      Void ratio
      void=void+(1.+void)*(DeltaE(1)+DeltaE(2)+DeltaE(3))
C      Type of model
      STATEV(1)=model
C      Void ratio
      STATEV(2)=void
C      Atmospheric mean pressure
      STATEV(3)=patm
C      Initial Shear modulus
      STATEV(4)=G0
C      Atmospheric mean pressure
      STATEV(5)=kappa
C -----
      RETURN
      END
```

UMAT syntax

Subroutine Initial:

```

C -----
      SUBROUTINE Initial(NTENS, NDI, NSHR, T, STRESS, DeltaE, DSTRAN)
      integer NTENS, NDI, NSHR, i, j, k, l
      double precision T(6), ELMOD(6,6), STRESS(NTENS),
1 DSTRAN(NTENS), DeltaE(6)
C      Stress and delta strain in a 6-vector
      T=0.0d0
      T(1:ndi)=STRESS(1:ndi)
      T(4:3+NSHR)=STRESS(ndi+1:NTENS)
      DeltaE=0.0d0
      DeltaE(1:ndi)=DSTRAN(1:ndi)
      DeltaE(4:3+NSHR)=DSTRAN(ndi+1:NTENS)
      Return
      END SUBROUTINE Initial
C -----

```

UMAT syntax

Subroutine Solution:

```

C -----
C -----
      SUBROUTINE Solution(NTENS, NDI, NSHR, T, STRESS, ELMOD, DDSDE)
      integer NTENS, NDI, NSHR
      double precision T(6), ELMOD(6,6), STRESS(NTENS),
1 DDSDE(NTENS,NTENS)
C -----
      STRESS(1:ndi)=T(1:ndi)
      STRESS(ndi+1:NTENS)=T(4:NSHR)

```

UMAT syntax

Subroutine Solution (continuation):

```
      do i=1,ndi
        do j=1,ndi
          ddsdde(i,j)=ELMOD(i,j)
        enddo
      enddo
do i=ndi+1,ndi+nshr
  do j=1,ndi
    ddsdde(i,j)=ELMOD(i,j)
  enddo
enddo
do i=1,ndi
  do j=ndi+1,ndi+nshr
    ddsdde(i,j)=ELMOD(i,j)
  enddo
enddo
do i=ndi+1,ndi+nshr
  do j=ndi+1,ndi+nshr
    ddsdde(i,j)=ELMOD(i,j)
  enddo
enddo
END SUBROUTINE Solution
```

Modified CAM-CLAY MCC plasticity model

MCC plasticity model

Yield surface:

$$f(q, p) := \frac{q^2}{M^2} + p(p - p_c) \quad (10)$$

where M is the material parameter which defines the slope of the critic state line, p_c the overconsolidated mean stress and q the deviator stress. see [Schofield et.al., 1968].

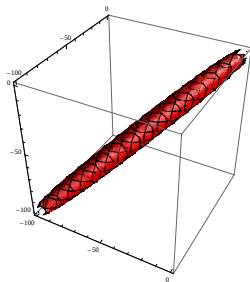


Figure: CAM CLAY surface

Hardening law

The hardening law is taken by the p_c .

$$\dot{p}_c = \vartheta p_c \dot{\epsilon}_v^p, \quad \dot{\epsilon}_v^p = \text{tr}(\dot{\epsilon}^p), \quad \vartheta = \frac{1 + e}{\lambda^* - \kappa^*} \quad (11)$$

where

- ϵ_v^p the plastic volumetric strain
- ϵ^p the plastic strain
- e the void ratio.
- λ^* the compression index (parameter).
- κ^* the swelling index (parameter).

Associative flow rule

$$\frac{\partial f(p, q)}{\partial \mathbf{T}} = \frac{\partial f}{\partial p} \frac{\partial p}{\partial \mathbf{T}} + \frac{\partial f}{\partial q} \frac{\partial q}{\partial \mathbf{T}} \quad (12)$$

Summarizing,

$$\mathbf{D}^p = \gamma \left[(2p - p_c) \frac{1}{3} \mathbf{1} + \sqrt{\frac{3}{2}} \frac{2q}{M^2} \vec{\mathbf{T}}^* \right] \quad (13)$$

being,

- $\mathbf{D}^p$ the rate of plastic strain tensor.
- $\vec{\mathbf{T}}^* = \frac{\mathbf{T} - \frac{1}{3} \mathbf{1}}{\| \mathbf{T} - \frac{1}{3} \mathbf{1} \|}$
- $\mathbf{1}$ the unit second order tensor.

Parameters:

- 1 λ^* Compression index
- 2 κ^* Swelling index.
- 3 p_{c0} Initial mean overconsolidated stress.
- 4 M Critical state line slope.
- 5 ν POISSON ratio.

State variables

- 1 e void ratio.
- 2 ε_{11}^p plastic strain component 11.
- 3 ε_{22}^p plastic strain component 22.
- 4 ε_{33}^p plastic strain component 33.
- 5 ε_{12}^p plastic strain component 12.
- 6 ε_{13}^p plastic strain component 13.
- 7 ε_{23}^p plastic strain component 23.
- 8 p_c mean overconsolidated stress.

Implicit Integration

See [Borja, R. I. et.al., 1990] for implicit integration. Some details of the integration scheme:

- Implicit integration using the closest point projection. A return mapping algorithm is used.
- The void ratio e is assumed to be constant in an integration increment.
- 2 nested NEWTON-RAPHSON algorithm needed in order to find the consistency parameter $\Delta\gamma$ and the mean consolidation stress p_c .
- A consistent Jacobian is used in order to preserve the asymptotic rate of quadratic global convergence[Borja, R. I. et.al., 1990].

The following frames presents the implicit integration scheme for the MCC model. Open the archive CamClay.f and follow the frames.

Implicit Integration

Over the plastic range,

$$P_c(p_{c,n+1}) = p_{c,n} \exp \left[\vartheta \Delta \gamma \left(\frac{2p_{n+1}^{tr} - p_{c,n+1}}{1 + 2\Delta \gamma k} \right) \right] - p_{c,n+1} = 0 \quad (14)$$

and

$$\frac{\partial f_{n+1}}{\Delta \gamma_{n+1}} = \frac{\partial f}{\partial p} \frac{\partial p}{\partial \Delta \gamma} + \frac{\partial f}{\partial q} \frac{\partial q}{\partial \Delta \gamma} + \frac{\partial f}{\partial p_c} \frac{\partial p_c}{\partial \Delta \gamma} \bigg|_{n+1} \quad (15)$$

where,

$$\frac{\partial p_{n+1}}{\Delta \gamma_{n+1}} = -k \frac{2p - p_c}{1 + (2k + \vartheta p_c) \Delta \gamma} \bigg|_{n+1} \quad (16)$$

$$\frac{\partial q_{n+1}}{\Delta \gamma_{n+1}} = - \frac{q}{\Delta \gamma + M^2/6G} \bigg|_{n+1} \quad (17)$$

$$\frac{\partial p_{c,n+1}}{\Delta \gamma_{n+1}} = \vartheta p_c \frac{(2p - p_c)}{1 + (2k + \vartheta p_c) \Delta \gamma} \bigg|_{n+1} \quad (18)$$

Implicit Integration

So, 2 nested Newton algorithm should be implemented.

- ① a trial elastic step is made.
- ② if $f < 0$ then the elastic step is ok.
- ③ if $f \geq 0$ a plastic corrector step should be implemented.
 - A global NEWTON algorithm for $f(q, p, p_c, \Delta\gamma) = 0$ is made.
 - a local NEWTON algorithm is made for p_c .
 - q and p are computed.
 - $\Delta\gamma$ is computed.
- ④ After q, p and $\Delta\gamma$ are computed, the plastic strains are computed.
- ⑤ Stress $\mathbf{T}$ is computed.
- ⑥ the Jacobian $[\mathbb{J}] = [\text{DDSDDE}]$ is computed.

Implicit Integration

Definition of constants

$$G = \frac{E}{2(1 + \nu)} \quad k = \frac{E}{3(1 - 2\nu)}$$

$$[1^a] = [1_a] = [1 \quad 1 \quad 1 \quad 0 \quad 0 \quad 0]^T$$

$$[C^{e,ab}] = \begin{bmatrix} \bar{\lambda} + 2G & \bar{\lambda} & \bar{\lambda} & 0 & 0 & 0 \\ & \bar{\lambda} + 2G & \bar{\lambda} & 0 & 0 & 0 \\ & & \bar{\lambda} + 2G & 0 & 0 & 0 \\ & & & G & 0 & 0 \\ & sym & & & G & 0 \\ & & & & & G \end{bmatrix}$$

$$\text{with } \bar{\lambda} = k - \frac{2}{3}G, (G = \mu)$$

Implicit Integration

Definition of constants (continuation)

$$[I^{ab}] = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ & 1 & 0 & 0 & 0 & 0 \\ & & 1 & 0 & 0 & 0 \\ & & & \frac{1}{2} & 0 & 0 \\ & & & & \frac{1}{2} & 0 \\ & & sym & & & \frac{1}{2} \end{bmatrix}$$

$$[I^{dev,ab}] = \begin{bmatrix} \frac{2}{3} & -\frac{1}{3} & -\frac{1}{3} & 0 & 0 & 0 \\ & \frac{2}{3} & -\frac{1}{3} & 0 & 0 & 0 \\ & & \frac{2}{3} & 0 & 0 & 0 \\ & & & \frac{1}{2} & 0 & 0 \\ & & & & \frac{1}{2} & 0 \\ & & sym & & & \frac{1}{2} \end{bmatrix}$$

$$[I_b^{dev,a}] = \begin{bmatrix} \frac{2}{3} & -\frac{1}{3} & -\frac{1}{3} & 0 & 0 & 0 \\ & \frac{2}{3} & -\frac{1}{3} & 0 & 0 & 0 \\ & & \frac{2}{3} & 0 & 0 & 0 \\ & & & 1 & 0 & 0 \\ & & & & 1 & 0 \\ & & sym & & & 1 \end{bmatrix}$$

Implicit Integration

Definition of initial conditions for state variables p_c, ε^p and stress $\mathbf{T}$.

Make a trial elastic step

$$[T_{n+1}^{tr,a}] = [T_{n+1}^a] + [C^{e,ab}][\Delta\varepsilon_{n+1,b}]$$

$$\| [T_{n+1}^{tr,a}] \| = \text{NORM}[T_{n+1}^{tr,a}]$$

$$p_{n+1}^{tr} = \frac{1}{3} \text{TRACE} [[T_{n+1}^{tr,a}]]$$

$$[T_{n+1}^{*,tr,a}] = [I^{dev,a}_b][T_{n+1}^{tr,b}]$$

$$\| [T_{n+1}^{*,tr,a}] \| = \text{NORM} [[T_{n+1}^{*,tr,a}]]$$

$$q_{n+1}^{tr} = \sqrt{\frac{3}{2}} \| [T_{n+1}^{*,tr,a}] \|$$

Implicit Integration

Check the yielding function.

$$f_{n+1}^{tr} = \frac{(q_{n+1}^{tr})^2}{M^2} + p_{n+1}^{tr}(p_{n+1}^{tr} - p_{c,n+1})$$

if $f_{n+1}^{tr} < 0$, trial elastic step is ok!. Then,

$$[T_{n+1}^a] = [T_{n+1}^{tr,a}]$$

$$p_{n+1} = p_{n+1}^{tr}$$

$$q_{n+1} = q_{n+1}^{tr}$$

$$p_{c,n+1} = p_{c,n}$$

$$[\varepsilon_{n+1,a}^p] = [\varepsilon_{n,a}^p]$$

$$[C^{ep,ab}] = [C^{e,ab}]$$

$$e_{n+1} = e_n + (1 + e_n)\text{TRACE}[\Delta\varepsilon_{n+1,b}]$$

Implicit Integration

if $f_{n+1}^{tr} \geq 0$, then plastic corrector step, define initial values and tolerances for iterations, and maximum number of iterations,

$$\Delta\gamma_{n+1} = 0$$

$$e_{n+1} = e_n + (1 + e_n)\text{TRACE}[\Delta\varepsilon_{n+1,b}]$$

$$\vartheta_{n+1} = (1 + e_{n+1})/(\lambda^* + \kappa^*)$$

$$\text{Tol}_f := 10^{-7} p_{c,n+1} \quad (\text{suggestion})$$

$$\text{TolP}_c := 10^{-7} \| [T_{n+1}^{*,tr,a}] \| \quad (\text{suggestion})$$

$$\text{max} = 50 \quad (\text{suggestion})$$

Implicit Integration

Making NEWTON AND RAPHSON algorithm for p_c

┐ for $i = 1$, until $i = \max$

┐ for $j = 1$, until $j = \max$

$$P_c = p_{c,n} \exp \left[\vartheta_{n+1} \Delta \gamma_{n+1} \left(\frac{2p_{n+1}^{tr} - p_{c,n+1}}{1 + 2\Delta \gamma_{n+1} k} \right) \right] - p_{c,n+1} = 0$$

┐ if $\| P_c \| < \text{Tol}P_c$ exit for. else:

$$\frac{\partial P_c}{\partial p_{c,n+1}} = -p_{c,n} \exp \left[\vartheta_{n+1} \Delta \gamma_{n+1} \left(\frac{2p_{n+1}^{tr} - p_{c,n+1}}{1 + 2\Delta \gamma_{n+1} k} \right) \right] \frac{(\Delta \gamma_{n+1})(\vartheta_{n+1})}{2k\Delta \gamma_{n+1} + 1} - 1$$

$$\underbrace{p_{c,n+1}}_{atj+1} = \underbrace{p_{c,n+1}}_{atj} - \frac{P_c}{\frac{\partial P_c}{\partial p_{c,n+1}}}$$

┐ end for (j)

Implicit Integration

Making NEWTON AND RAPHSON for yielding function f

▮ calculate current p_{n+1} and q_{n+1}

$$p_{n+1} = \frac{p_{n+1}^{tr} + \Delta\gamma k p_{c,n+1}}{1 + 2\Delta\gamma k}$$

$$q_{n+1} = \frac{q_{n+1}^{tr}}{1 + 6G\Delta\gamma_{n+1}/M^2}$$

▮ calculate current yielding function f_{n+1}

$$f_{n+1} = \frac{q_{n+1}^2}{M^2} + p_{n+1}(p_{n+1} - p_{c,n+1}) \quad (19)$$

Implicit Integration

Making NEWTON AND RAPHSON for yielding function f (continuation)

if $\|f_{n+1}\| < \text{Tol}$ exit for. else:

$$\frac{\partial f_{n+1}}{\partial p_{n+1}} = 2p_{n+1} - p_{c,n+1}$$

$$\frac{\partial f_{n+1}}{\partial q_{n+1}} = 2q_{n+1}/M^2$$

$$\frac{\partial f_{n+1}}{\partial p_{c,n+1}} = -p_{n+1}$$

$$\frac{\partial p_{n+1}}{\partial \Delta\gamma_{n+1}} = -k \frac{2p - p_c}{1 + (2k + \vartheta p_c)\Delta\gamma} \Big|_{n+1}$$

$$\frac{\partial q_{n+1}}{\partial \Delta\gamma_{n+1}} = -\frac{q}{\Delta\gamma + M^2/6G} \Big|_{n+1}$$

$$\frac{\partial p_{c,n+1}}{\partial \Delta\gamma_{n+1}} = \vartheta p_c \frac{(2p - p_c)}{1 + (2k + \vartheta p_c)\Delta\gamma} \Big|_{n+1}$$

and...

Implicit Integration

Making NEWTON AND RAPHSON for yielding funciton f (continuation)

and...

$$\frac{\partial f_{n+1}}{\Delta \gamma_{n+1}} = \frac{\partial f}{\partial p} \frac{\partial p}{\partial \Delta \gamma} + \frac{\partial f}{\partial q} \frac{\partial q}{\partial \Delta \gamma} + \frac{\partial f}{\partial p_c} \frac{\partial p_c}{\partial \Delta \gamma} \bigg|_{n+1}$$

$$\underbrace{\Delta \gamma_{n+1}}_{ati+1} = \underbrace{\Delta \gamma_{n+1}}_{ati} - \frac{f_{n+1}}{\frac{\partial f_{n+1}}{\Delta \gamma_{n+1}}}$$

┐ end for (i)

Implicit Integration

Plastic strains and stresses

Plastic strains are calculated as follows,

$$[\vec{T}_{n+1}^{*,tr,a}] = \frac{[T_{n+1}^{*,tr,a}]}{\| [T_{n+1}^{*,tr,a}] \|}$$

$$[\vec{B}_{n+1,a}] = (2p_{n+1} - p_{c,n+1}) \frac{1}{3} [1_a] + \sqrt{\frac{3}{2}} \frac{2q_{n+1}}{M^2} [\vec{T}_{n+1}^{*,tr,a}]$$

$$[\Delta \varepsilon_{n+1,a}^p] = \Delta \gamma_{n+1} * [\vec{B}_{n+1,a}]$$

$$[\varepsilon_{a,n+1}^p] = [\varepsilon_{a,n}^p] + [\Delta \varepsilon_{n+1,a}^p]$$

▢ The stress is calculated according to the return algorithm scheme,

$$[T_{n+1}^{*,a}] = [T_{n+1}^{*,tr,a}] - [C^{e,ab}][\Delta \varepsilon_{n+1,b}^p]$$

Implicit Integration

Finally the consistent elastoplastic moduli is given by,

$$f_1 = 2G\sqrt{\frac{2}{3}} \frac{q_{n+1}}{\| \vec{T}_{n+1}^{*,tr,a} \|}$$

$$f_2 = \left[k(a_1 + a_2 d_1) - \frac{1}{3} 2G\sqrt{\frac{2}{3}} \frac{q_{n+1}}{\| \vec{T}_{n+1}^{*,tr,a} \|} \right]$$

$$f_3 = k(a_2 d_2)$$

$$f_4 = 2G\sqrt{\frac{2}{3}} (c_2 d_1)$$

$$f_5 = 2G \left[\sqrt{\frac{2}{3}} (c_1 + c_2 d_2) - \sqrt{\frac{2}{3}} \frac{q_{n+1}}{\| \vec{T}_{n+1}^{*,tr,a} \|} \right]$$

$$\begin{aligned} [C_{n+1}^{ep,ab}] &= f_1 [I^{ab}] + f_2 [1_a][1_a]^T + f_3 [1_a][\vec{T}_{n+1}^{*,tr,a}]^T + f_4 [\vec{T}_{n+1}^{*,tr,a}][1_a]^T \\ &+ f_5 [\vec{T}_{n+1}^{*,tr,a}][\vec{T}_{n+1}^{*,tr,a}]^T \end{aligned}$$

UMAT ready!! Response envelopes using INCREMENTAL DRIVER (see [Niemunis, 2008]).

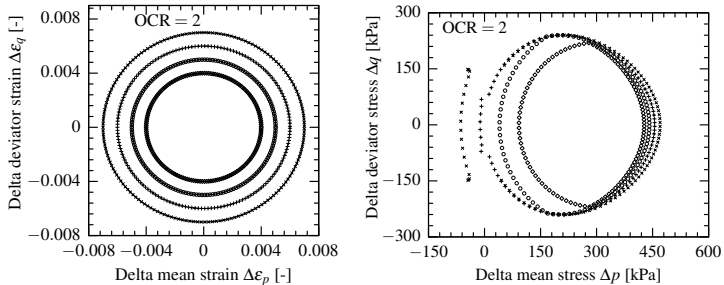


Figure: Response envelopes with MCC model

Hypoplasticity model

Constitutive model

- Done for granular materials.
- Assumes non-linear behaviour without an elastic range.
- Time-independent model formulated in a rate-form constitutive equation.
- $\overset{\circ}{\mathbf{T}} = \mathbf{h}(\mathbf{T}, e, \mathbf{D})$, being $\mathbf{D}$ the strain rate tensor and $\overset{\circ}{\mathbf{T}}$ the ZAREMBA-JAUMMAN co-rotational stress rate tensor.
- Model from [Wolffersdorff, V., 1996].
- BAUER's law for compression.
- MATSUOKA-NAKAI surface as the limit surface.

Constitutive model

$$\overset{\circ}{\mathbf{T}} = f_s [\mathbb{L} : \mathbf{D} + f_d \mathbf{N} \parallel \mathbf{D} \parallel] \quad (20)$$

$\mathbb{L}$ is the fourth order hypoelastic tensor and $\mathbf{N}$ is a second order tensor non-linear with

the stretching tensor $\mathbf{D}$.

$$\mathbb{L} = \frac{f_s}{\hat{\mathbf{T}} : \hat{\mathbf{T}}} a^2 \left[\left(\frac{F}{a} \right)^2 \mathbb{I} + \hat{\mathbf{T}} \otimes \hat{\mathbf{T}} \right] \quad (21)$$

$$\mathbf{N} = \frac{f_s}{\hat{\mathbf{T}} : \hat{\mathbf{T}}} a^2 \left[\frac{F}{a} \right] [\hat{\mathbf{T}} + \hat{\mathbf{T}}^*] \quad (22)$$

with,

$$\hat{\mathbf{T}} = \frac{\mathbf{T}}{\text{Tr}[\mathbf{T}]} \quad (23)$$

$$\hat{\mathbf{T}}^* = \frac{\mathbf{T} - \text{Tr}[\mathbf{T}] \mathbf{1}}{\text{Tr}[\mathbf{T}]} \quad (24)$$

$$I_{ijkl} = \frac{1}{2} (\delta_{ik} \delta_{jl} + \delta_{il} \delta_{jk}) \quad (25)$$

Constitutive model

$$\overset{\circ}{\mathbf{T}} = f_s [\mathbb{L} : \mathbf{D} + f_d \mathbf{N} \parallel \mathbf{D} \parallel] \quad (26)$$

f_d is the density factor, f_e is the piktnotropy factor, and f_b is the barotropy factor: With,

$$f_e = \left(\frac{e_c}{e} \right) \quad (27)$$

$$f_d = \left(\frac{e - e_d}{e_c - e_d} \right)^\alpha \quad (28)$$

$$f_b = \left(\frac{e_{i0}}{e_{c0}} \right)^\beta \frac{h_s}{n} \frac{1 + e_i}{e_i} \left(\frac{-\text{tr} \mathbf{T}}{h_s} \right)^{1-n} \left[3 + a^2 - a\sqrt{3} \left(\frac{e_{i0} - e_{d0}}{e_{c0} - e_{d0}} \right) \alpha \right]^{-1} \quad (29)$$

$$f_s = f_e f_b \quad (30)$$

e_c , e_d and e_i are the critical, minimum and maximum void ratio according to the BAUER's law. h_s , n α and β are materials parameters. a is function of the critical friction angle φ_c .

BAUER's law for isotropic compression

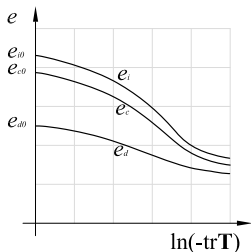


Figure: BAUER's law for isotropic compression

$$\frac{e_i}{e_{i0}} = \frac{e_c}{e_{c0}} = \frac{e_d}{e_{d0}} = \exp \left[- \left(- \frac{\text{tr} \mathbf{T}}{h_s} \right)^n \right] \quad (31)$$

MATSUOKA-NAKAI's limit surface

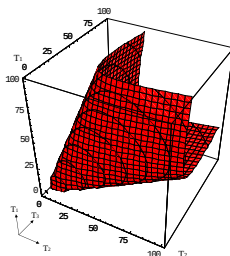
$$y_{M-N}(\mathbf{T}) = -\frac{I_1 I_2}{I_3} + \frac{9 - \sin^2(\varphi_c)}{-1 + \sin^2(\varphi_c)} = 0 \quad (32)$$

with,

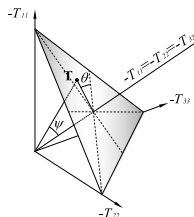
$$I_1 = \text{tr} \mathbf{T}$$

$$I_2 = \frac{1}{2}[\mathbf{T} : \mathbf{T} - (I_1)^2] \quad (33)$$

$$I_3 = \det(\mathbf{T})$$



MATSUOKA-NAKAI's limit surface



$$a = \frac{\sqrt{3}(3 - \sin(\varphi_c))}{2\sqrt{2}\sin(\varphi_c)} \quad (34)$$

$$F = \sqrt{\frac{1}{8} \tan^2(\psi) + \frac{2 - \tan^2(\psi)}{2 + \sqrt{2} \tan(\psi) \cos(3\theta)}} - \frac{1}{2\sqrt{2}} \tan(\psi) \quad (35)$$

$$\tan(\psi) = \sqrt{3} \parallel \hat{\mathbf{T}}^* \parallel \quad (36)$$

$$\cos(3\theta) = -\sqrt{6} \frac{\text{tr}(\hat{\mathbf{T}}^{*3})}{[\text{tr}(\hat{\mathbf{T}}^*)]^{3/2}} \quad (37)$$

Explicit implementation

- ABAQUSTM establishes a co-rotational frame for UMATs. It is not necessary to make correction by sc Zaremba-Jaumann $\overset{\circ}{\mathbf{T}}$ proposal.
- Eulers forward method with not error control implemented.
- The Jacobian $\mathbb{J}$ is computed according to [Fellin, W. et.al. 2002].
- Tensor $\mathbf{D}$ is calculated using the approximation $\mathbf{D} = \frac{\Delta \boldsymbol{\varepsilon}}{\Delta t}$

Jacobian by numeric differentiation

Jacobian:

$$\mathbb{J} = \frac{\partial \Delta \mathbf{T}}{\partial \Delta \boldsymbol{\varepsilon}} \quad (38)$$

The hypoplastic law establishes the following relation for the rate of $\mathbf{T}$:

$$\frac{d}{dt} \mathbf{T} = \mathbf{h}(\mathbf{T}, \mathbf{D}, \mathbf{Q}) \quad (39)$$

where $\mathbf{Q}$ denotes the additional state variables.

Introducing,

$$B_{ij,kl} = \frac{\partial T_{ij}}{\partial D_{kl}} = \Delta t \cdot \frac{\partial \Delta T_{ij}}{\partial \Delta \varepsilon_{ij}} \quad (40)$$

This means that an approximation of the Jacobian is:

$$\mathbb{J} \simeq \frac{B_{ij,kl}}{\Delta t} \quad (41)$$

Jacobian by numeric differentiation

Developing numerically $B_{ij,kl}$:

The following is a numeric differentiation of $\frac{d}{dt}\mathbf{B}_{ij}$:

$$\frac{d}{dt}\mathbf{B}_{ij} = \frac{1}{\vartheta} [\mathbf{h}(\mathbf{T} + \vartheta\mathbf{B}_{ij}, \mathbf{D} + \vartheta\mathbf{V}_{ij}, \mathbf{Q} + \vartheta\mathbf{G}_{ij}) - \mathbf{h}(\mathbf{T}, \mathbf{D}, \mathbf{Q})] \quad (42)$$

where

$$\mathbf{V}_{ij} = \delta_{ik}\delta_{jl} \quad (43)$$

for example,

$$[\mathbf{V}_{11}] = \begin{bmatrix} 1 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \quad (44)$$

and

$$\mathbf{G}_{ij} = \frac{\partial \mathbf{Q}}{\partial \mathbf{D}} \quad (45)$$

Jacobian by numeric differentiation

Arrange of the super-vector solution

For programming the super-vector $\mathbf{y}$ will be employed:

$$\begin{aligned}
 [y_1, y_2, \dots, y_6]^T &= [T_{11}, T_{22}, T_{33}, T_{12}, T_{13}, T_{23}]^T \\
 [y_7, \dots, y_{42}]^T &= [B_{11,11}, B_{11,22}, \dots, B_{23,23}]^T \quad (36 \text{ components}) \\
 [y_{43}, y_{44}]^T &= [\varphi_m, e]^T \\
 [y_{45}, \dots, y_n]^T &= [G_{e,11}, G_{\varphi_m,11}, G_{e,22}, G_{\varphi_m,22}, \dots, G_{e,23}, G_{\varphi_m,23}]^T
 \end{aligned} \tag{46}$$

and a rate super-vector $\dot{\mathbf{y}}$ such that, an EULER forward integration can be computed as:

$$\mathbf{y}_{n+1} = \mathbf{y}_n + \Delta t \cdot \dot{\mathbf{y}} \tag{47}$$

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